

Experimental and analytical analysis of racking resistance of partially  
anchored timber frame walls

Non Peer-reviewed author version

STEENSELS, Rik; DE PROFT, Kurt & VANDOREN, Bram (2017) Experimental and analytical analysis of racking resistance of partially anchored timber frame walls. In: De Proft, Kurt; Brebbia, C.A.; Connor, J. (Ed.). International Journal of Computational Methods and Experimental Measurements, WIT Press / Computational Mechanics, p. 872-883.

DOI: 10.2495/CMEM-V5-N6-847-857

Handle: <http://hdl.handle.net/1942/25019>

# EXPERIMENTAL AND ANALYTICAL ANALYSIS OF RACKING RESISTANCE OF PARTIALLY ANCHORED TIMBER FRAME WALLS

R. STEENSELS<sup>1</sup>, K. DE PROFT<sup>2</sup> AND B. VANDOREN<sup>1</sup>

<sup>1</sup>Hasselt University – Faculty of Engineering Technology, Martelarenlaan 42, 3950 Diepenbeek, Belgium

<sup>2</sup>WOOD.BE – Belgian Institute for Wood Technology, Hof ter Vleestdreef 3, 1070 Brussels, Belgium

## ABSTRACT

The resistance to horizontal loads provided by timber constructions is determined by the racking resistance of the timber frame walls within the structure. In Eurocode 5 (EN 1995-1-1), two methods are described to assess the racking resistance of these structural elements. Method A refers to a mechanical model while method B is empirically based and therefore less attractive. When using method A, the full anchorage of the leading stud is needed. Moreover contributions of wall panels with openings in the racking resistance is neglected. In this paper, an experimental campaign studying the racking resistance of partially anchored walls with different wall and loading configurations is presented. The study shows that window and door openings lead to a reduction of the racking resistance of the wall depending on the size of the opening. Additionally, a comparison between the experimental data and several design methods for the assessment of the racking resistance of the wall panels is made.

*Keywords: Timber frame wall panels, racking resistance, experimental campaign*

## 1 INTRODUCTION

Correct assessment of the racking resistance of timber frame wall panels is of great importance during the structural analysis of timberstructures. Eurocode 5 (EN 1995-1-1, EC5) [1] provides two different methods for the calculation of the resistance to horizontal forces for timber frame wall panels. Method A is based on a mechanical method, while method B is an empirically based model. Method A forces the designer to apply full anchorage of the leading stud of the wall, necessitating the use of tie-downs to ensure proper anchorage. In turn, high reaction forces are induced at the bottom of the leading stud leading to an intricate design of the joint of this wall panel. Furthermore, EC5 does not allow for the inclusion of wall panels with openings in the analysis of the racking resistance. Any wall panel with a door or window opening is to be neglected in the assessment of the total racking resistance of the full wall.

Modern timber constructions often involve more open floor plans with a considerable amount of openings in the wall panels. A more advanced method for the assessment of the racking resistance can therefore optimise the design of these structures and minimise the material usage. In this paper, an experimental campaign is presented for the assessment of the racking resistance of partially anchored, wooden framed walls. The total racking resistance of a range of walls consisting of multiple wall panels, with and without openings and with different load characteristics is investigated and compared to different analytical methods available in the literature.

## 2 DESIGN MODELS FOR PARTIALLY ANCHORED WALLS

As previously mentioned, EC5 does not allow for the calculation of the racking resistance of partially anchored walls. More recently, several analytical models have been developed which also allow for the assessment of the racking resistance of partially anchored walls. 2 models will be discussed in this paper. A perfectly plastic model suggested by Källsner and Girhammar [2, 3] and the simplified model suggested by De Proft [4].

### 2.1 Källsner and Girhammar model

The calculation of the racking resistance proposed by Källsner and Girhammar [2] is based on the static theorem which states that a load case satisfying the equilibrium conditions of the structure and yield condition of the applied materials is acceptable and conservative. The connectors are assumed to behave perfectly plastic in order to provide a lower bound value for the racking resistance. Furthermore, the fasteners are assumed to be distributed evenly along the perimeter of each individual wall panel. Consequently, the capacity of the connections can be expressed as a force per unit length:

$$f_p = \frac{F_y}{S} \quad (1)$$

where  $f_p$  is the capacity of the fasteners in N/mm,  $F_y$  is the yielding load of an individual fastener and  $S$  equals the spacing of the fasteners.

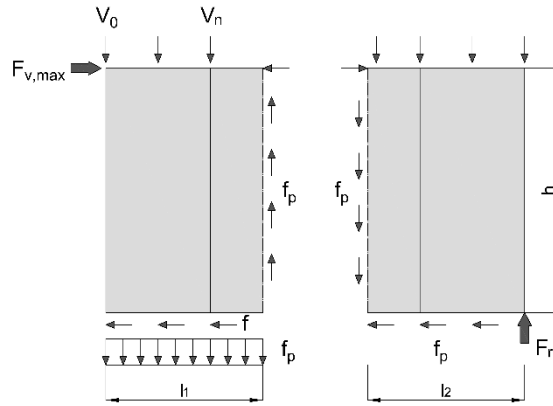


Figure 1: Källsner and Girhammar design model for walls without openings.

#### 2.1.1 Walls without openings

To assess the racking resistance of timber framed wall elements without openings, consider the partially anchored wall presented in figure 1. In the analysis procedure of Källsner and Girhammar [3] the wall is divided in two parts. In the first part, uplift of the vertical studs can occur and the forces are transferred from the frame structure to the substrate via the connections with the sheathing material. In the second part, the wall panels are assumed to be fully anchored. As a result, the forces are distributed to the substrate directly via the frame structure. The racking resistance ( $F_{v,max}$ ) of the wall can be determined using the following equations:

$$F_{v,max} = f_p \cdot l_{eff} \quad (1)$$

$$l_{\text{eff}} = \left( \frac{l_1}{2h} + \frac{V_{\text{eq}}}{f_p h} \right) l_1 + l_2 \quad (2)$$

$$V_{\text{eq}} = \sum_{i=0}^n \left( \frac{l_1 - x_i}{l_1} V_i \right) \quad (3)$$

In these equations,  $l_{\text{eff}}$  describes the length of the wall which will effectively contribute to the horizontal strength.  $l_1$  and  $l_2$  are, respectively, the length of the partially anchored and fully anchored wall-sections.  $h$  represents the total height of the wall.  $V_{\text{eq}}$  is the equivalent vertical load on the leading stud of the wall and  $x_i$  equals the length between the leading stud and the vertical stud to which an additional vertical load  $V_i$  is applied.

### 2.1.2 Walls with openings

When an opening is introduced in the wall, the same basic strategy is applied. The wall is divided into segments left and right of each opening (figure 2). Each part is evaluated comparable to the method applied for walls without openings. However, additional contributions to the total racking resistance resulting from the panels with the openings are also included in the analysis. An example of such a wall is presented in figure 2. Figures 3 and 4 present the force distribution on which the design model is based.

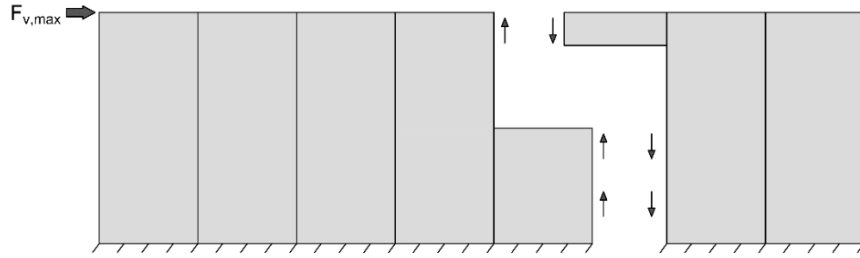


Figure 2: Wooden frame wall with opening.

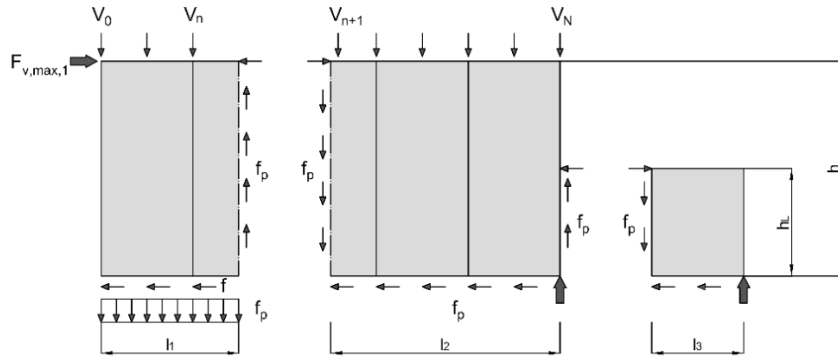


Figure 3: Källsner and Girhammar design model for wall panels left side of the opening.

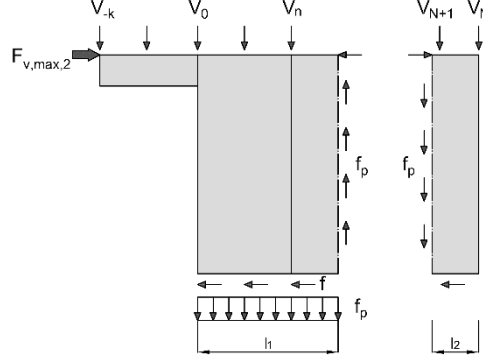


Figure 4: Källsner and Girhammar design model for wall panels right side of the opening.

The racking resistance of the wall segment to the left of the opening is computed using the following equations:

$$l_1 = h \left( 1 - \frac{\sum_{i=0}^N V_i}{f_p h} \right) \quad (4)$$

$$l_2 = l_l - l_1 \quad (5)$$

$$V_{eq} = \sum_{i=0}^N \left( \frac{l_1 - x_i}{l_1} V_i \right) \quad (6)$$

$$l_{eff,1} = \left( \frac{l_1}{2h} + \frac{V_{eq}}{f_p h} \right) l_1 + l_2 + \frac{f_l h_l}{f_p h} \lambda l_3 \quad (7)$$

$$\lambda = \min \begin{cases} \frac{f_l h}{f_p l_3 h_l} \left[ \left( 1 - \frac{l_1}{2h} - \frac{V_{eq}}{f_p h} \right) l_1 + \frac{n_y F_y}{f_p} \right] \\ \frac{f_l}{f_p} \frac{2h}{l_3 (h - h_l)} \left[ \left( \frac{l_1}{2h} - \frac{V_{eq}}{f_p h} \right) l_1 + l_2 \right] \\ \frac{f_l b_{p1}}{f_p l_3} \end{cases} \quad (8)$$

$$F_{v,max,1} = f_p \cdot l_{eff,1} \quad (9)$$

with  $l_l$  representing the length of the wall segment to the left of the opening,  $f_l$  equals the capacity of the connections [N/mm] at the wall panel below the opening,  $h_l$  and  $l_3$  is the height and length of the wall panel below the opening,  $n_y$  and  $F_y$  equal the number and capacity of the connections attached to the upper horizontal beam, and  $b_{p1}$  is the width of a single wall panel.

The wall segment to the right of the opening is evaluated as follows:

$$l_1 = h \left( 1 - \frac{\sum_{i=0}^N V_i}{f_p h} \right) \quad (10)$$

$$V_{eq} = \sum_{i=0}^N \left( \frac{l_1 - x_i}{l_1} V_i \right) \quad (11)$$

$$l_{eff,2} = \left( \frac{l_1}{2h} + \frac{V_{eq}}{f_p h} \right) l_1 + l_2 \quad (12)$$

$$F_{v,max,2} = f_p \cdot l_{eff,2} \quad (13)$$

$$F_{v,max,tot} = F_{v,max,1} + F_{v,max,2} \quad (14)$$

## 2.2 De Proft model

The analyses method for partially anchored walls suggested by De Proft [4] is also based on the static theorem (lower bound) and starts from the load distribution of a single wall panel (figure 5).

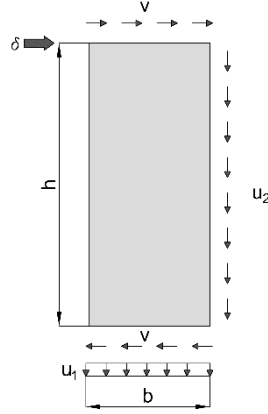


Figure 5: Load distribution of a partially anchored wall panel under horizontal load

The maximum capacity in the connections at the bottom of the panel is described by:

$$f_p = \frac{F_y}{s} = \sqrt{u_1^2 + v^2} \quad (15)$$

From equation 15 and the rotational equilibrium of the panel, the racking resistance  $F_{v,max}$  of the wall is determined to be equal to

$$F_{v,max} = \frac{F_y \cdot b}{s \cdot \sqrt{1 + \left( \frac{2h}{b} \right)^2}} \quad (16)$$

The calculation of the racking resistance alters when the wall is constructed of multiple wall panels. For these walls, De Proft provides a complex and simplified method for the assessment of the racking resistance. The complex method involves a more exact computation of the racking resistance. The anchoring force  $u_2$  in the end stud of the previous wall panel is evaluated together with the racking resistance of the observed wall

panel using a system of equilibrium equations. However, a simplified method is also suggested in which equation 16 is applied and the width  $b$  of the entire wall instead of a single wall panel is used. It is stated that this approach provides an efficient assessment of the racking resistance with an acceptable rate of error in comparison with the exact solution [4].

The method proposed by De Proft only evaluates the racking resistance for wall panels without openings. A panel ratio based on the work of Sugiyama [5] and Yasumura [6] is applied in order to take the influence of openings into account. This panel ratio ( $r$ ) is developed using an experimental and numerical campaign and re-evaluates the racking resistance of the wall taking into account the geometrical properties of the opening ( $F_{v,max,o}$ ). The following equations are applied:

$$F_{v,max,o} = Q \cdot F_{v,max} \quad (17)$$

$$Q = \begin{cases} r & \text{Fully anchored} \\ \frac{r}{2-r} & \text{Partially anchored} \end{cases} \quad (18)$$

$$r = \frac{1}{1+\frac{\alpha}{\beta}} \quad (19)$$

$$\alpha = \frac{a_o \cdot b_o}{L \cdot h} \quad (20)$$

$$\beta = \frac{\sum b_{p2}}{L} \quad (21)$$

Where  $Q$  represents the racking resistance ratio.  $r$  equals the panel ratio.  $\alpha$  and  $\beta$  are dimensionless parameters taking into account the width ( $a_o$ ) and height ( $b_o$ ) of the openings, the total length of the wall ( $L$ ), height of the wall ( $h$ ) and the sum of the width of the wall panels without openings ( $b_{p2}$ ).

### 3 EXPERIMENTAL CAMPAIGN

#### 3.1 Test set-up

An experimental campaign has been performed investigating the racking resistance of partially anchored wooden frame wall panels [7]. 11 different panel configurations (figure 6) are tested. Additionally, a different load configuration is applied to 3 panel configurations leading to the assessment of the total racking resistance of 14 test specimens.

The walls are constructed on a 1:2 scale. The height of the elements is 1400 mm, the width of each wall panel is 600 mm. The different wall configurations consist of up to 4 wall panels so the width of the walls ranges from 600 to 2400 mm. The timber used in the timber frame is classified C24. However, the density of the timber is higher than the density expected for C24. The cross section of the beam elements measure 50 mm x 70 mm. The distance between each vertical stud is 300 mm. The sheathing material consists of particle boards -type P5 with a thickness of 12 mm. The wall panels are connected to the wooden frame structure using 1.70 mm x 1.88 mm staples with a length of 50 mm. The spacing

between the connectors is 150 mm for the outer studs of each individual panel. The spacing in the middle stud is 300 mm. Door openings have a height of 1075 mm. The height of window openings is 575 mm. Each wall is anchored to the substrate using 3 screws along the first 300 mm of the bottom bar. In the remainder of the bottom rule, 2 screws every 300 mm are provided for additional anchorage to the substrate.

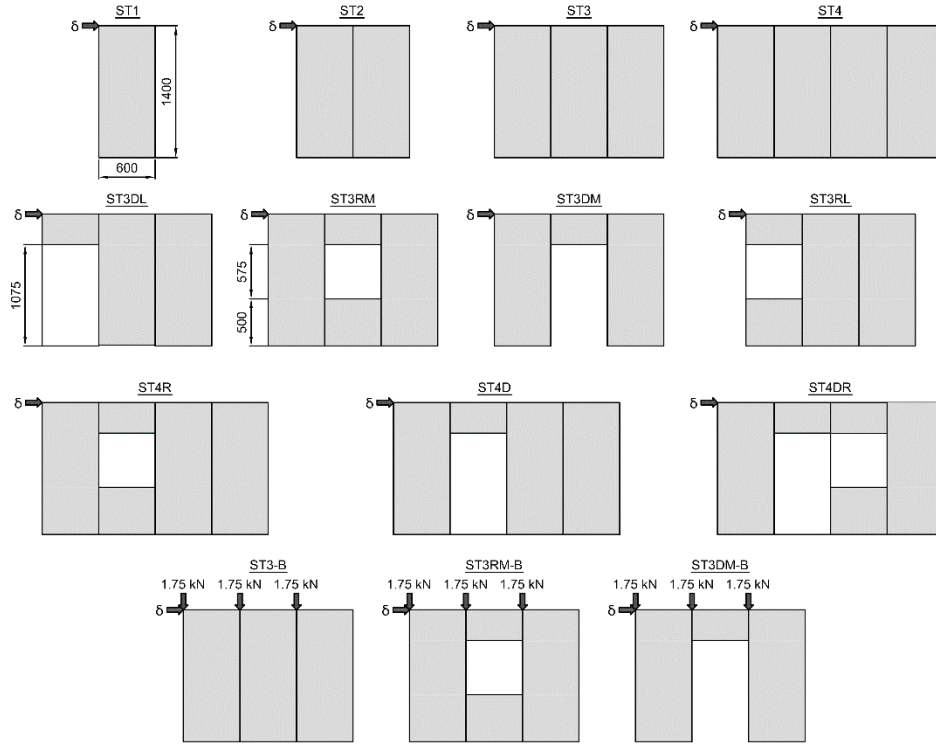


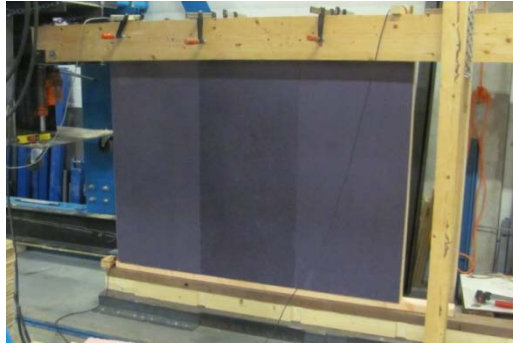
Figure 6: Wall configurations of test specimens.

The racking resistance of each wall configuration is determined in accordance with EN 594 [8] and equals the maximum registered horizontal force during a horizontal displacement ( $\delta$ ) of 100 mm. The horizontal displacement is applied at the top left corner of the test specimen and measured at the top and bottom bar of the wall. The vertical displacement of the leading stud is also measured. For some wall configurations (ST3-B, ST3RM-B, ST3DM-B), an additional vertical load of 1.75 kN is applied to the leading stud of each wall panel.

Figure 7 presents the experimental set-up of the ST3 and ST3-B wall configurations. Guiding beams have been placed at the top of the test set-up at both sides of the test specimen (figure 7a). Rollers are applied between the guiding beams and the wall specimens to ensure that no out of plane movement occurs during the test (figure 7b). Figures 7c and 7d present the load configuration of the ST3-B specimen. A vertical force of



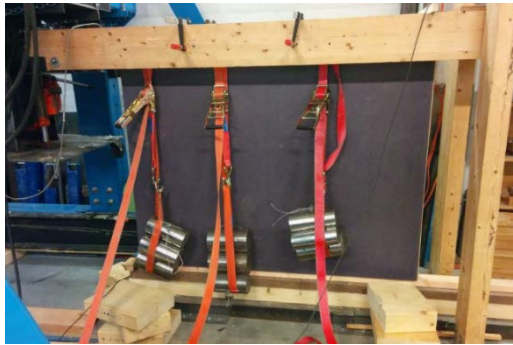
1.75 kN is applied to the leading stud of each individual wall panel by applying a strap with weights over the wall at the location of the studs.



(a) ST3 test set-up, front view



(b) ST3 test set-up, side view



(c) ST3-B test set-up, front view



(d) ST3-B test set-up, side view

Figure 7: Experimental test set-up.

### 3.2 Experimental results

The load-displacement diagrams resulting from the tests are presented in figure 8. Figure 8a compares the racking resistance of wall specimens with increasing width and without openings. As expected, the larger wall specimens provide a larger total racking resistance.

Figure 8b shows the load-displacement diagrams of the wall specimens constructed of 3 wall panels. A clear influence of the presence of an opening on the racking resistance of the specimen can be seen. The racking resistance of the walls with 3 panels drops to 60% of the original strength when a door opening is introduced in the wall. A window opening causes a decline of the racking resistance to 85% compared to a wall without openings. Furthermore, figure 8b shows that the location of the door or window opening has a negligible influence on the racking resistance.

Figure 8c shows that the behaviour of the test specimen consisting of 4 wall panels. The racking resistance for the wall panels with a door and window opening drops to values of 57 and 85% respectively. This is very similar to influence of the openings in the specimen

with 3 wall panels. Remarkably, the racking resistance of the ST4D and ST4D+R specimens is also very similar even though the total size of the opening in the ST4D+R test specimen is considerably larger.

Figure 8d shows the influence of a vertical load on the racking resistance of a partially anchored wooden frame wall. It is apparent that this vertical load provides additional anchorage to the leading studs of the individual wall panels. Therefore, a higher racking resistance is reached for these wall specimens.

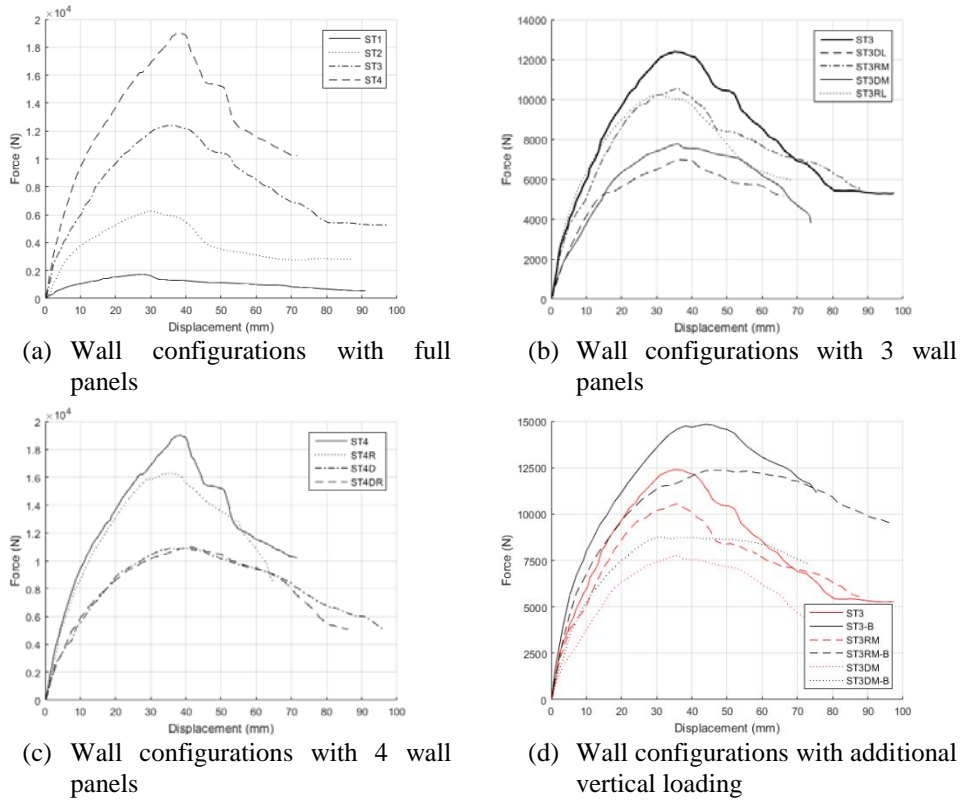


Figure 8: Force-displacement diagrams of wall configurations.

#### 4 COMPARISON BETWEEN EXPERIMENTAL AND ANALYTICAL MODELS

Figure 9 presents the comparison between the experimental data and the results of the plastic model of Källsner and Girhammar, and the simplified model of De Proft combined with the panel ratio of Yasumura to take into account the openings in the wall panels. It can be seen that the combination of the simplified model proposed by De Proft in combination with the panel ratio found in the work of Yasumura provides better agreement with the experimental readings in comparison with the model of Källsner and Girhammar. However, considerable discrepancies between the experimental and analytical data can still be distinguished for some test specimens (e.g. ST3-RL, ST4D+R). Furthermore, the analytical

methods lead to both over- and underestimated values of the racking resistance. Therefore, the analytical methods do not always lead to conservative design.

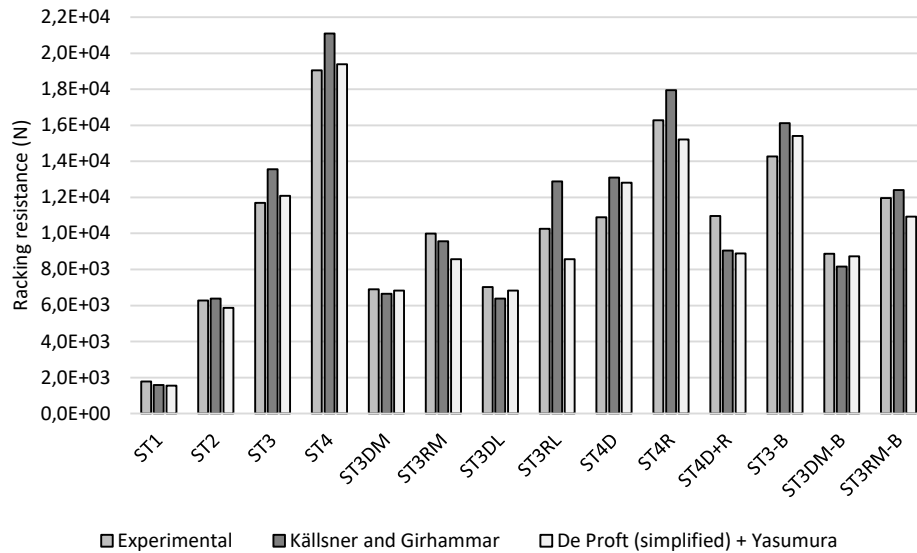


Figure 9: Comparison between experimental and analytical data.

## 5 CONCLUSIONS

This paper presents an analytical and experimental campaign for the evaluation of the racking resistance of partially anchored wooden frame walls. Two analytical methods for the assessment of the racking resistance, found in the available literature, are presented and compared to experimental data of 14 test specimens. Both the influence of openings in the wall panels and presence of additional vertical loads on the studs of wooden frame are included. It is concluded that the openings in wall panels decrease the total racking resistance of the walls based on their size. The location of the opening was found to have a negligible influence. The presence of a vertical load on the studs of the wooden frame causes additional anchorage of the wall panels. As a result the racking resistance will increase. Considering the analytical assessment of the racking resistance, it is found that the simplified method of De Proft combined with the panel ratio of Yasumura provides a better agreement with the experimental data compared to the method suggested by Källsner and Girhammar. However, considerable differences between experimental and analytical data can still be identified for some test specimens.

## REFERENCES

- [1] EN 1995-1-2, Design of timber structures: part 1-1: General – Common rules and rules for buildings 2005.
- [2] Källsner B. and Girhammar U. A., *A plastic lower bound method for design of wood-framed shear walls*, Proceedings 8<sup>th</sup> World Conference on Timber engineering, Finland, June 14-17, 2004

- [3] Källsner B. and Girhammar U. A., *A plastic design method for incompletely anchored wood-framed wall diaphragms*, 9<sup>th</sup> World Conference of Timber Engineering, Portland Oregon USA, August 6-10, 2006
- [4] De Proft K., *Stability of timber constructions: derivations of typical solutions part II: Horizontal stability of timber frame walls*, Wood.be, 2013
- [5] Sugiyama H., Matsumoto T., *Empirical equations for the estimation of racking strength of a plywood shear wall with openings*, Summary of Technical Papers, Annual Meetings, Trans of A.I.J. Japan, 1994
- [6] Yasamura, M., *Racking resistance of panel-sheathed shear walls with opening*, Department of Environment and Forest Resources Science, Shizuoka University. Japan, 2010
- [7] Reweghs K., *Schrankweerstand van houtskeletwanden: invloed van raam- en deuropeningen*, Master thesis, Hasselt University, 2015
- [8] Bureau for Standardisation, *NBN EN 594: Timber structures – test methods – Racking strength and stiffness of timber frame wall panels*, Brussels, 2011